

Customer: Best customer – Project reference: Building of the Century

Executed by Vincent Juhel Ouaip, on Friday, 22 March 2019.

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Global data

Building address: 21 Rue Général Bérenger, 06800 Cagnes-sur-Mer, France

Altitude: 19 m Gravity: 9.805 N/kg

Eurocodes standards :

- basis : EN 1990 (03/2003) + FR NA (12/2011)
(Consequences class CC2 = Medium consequence for loss of human life, economic, social or environmental consequences considerable.)
- snow loads : EN 1991-1-3 (07/2003) + FR NA (05/2007)
- wind actions : EN 1991-1-4 (2005) + FR NA (03/2008)
- seismic actions : EN 1998-1 (12/2004) + FR NA (12/2013)

Seism

Seism standard

The standard EN 1998-1 (12/2004) and its national annex FR NA (12/2013) define the design rules and design methods for seismic actions on buildings.

Legislative texts about seism

- Zoning: Art. D563-8-1, Décret N°2010-1255 (22/10/2010)
 - The construction is located at Cagnes-sur-Mer in a average seismicity zone (zone 4).
- Classification: Art. 2, Arrêté du 22/10/2010 modifié par Art.1, Arrêté du 15/09/2014

Reference peak ground acceleration

The intensity of the acceleration a_{gr} is function of the seismic zone. Its value of 1.6 m/s² is specified in Art. 4, Arrêté du 22/10/2010 modifié par Art.1, Arrêté du 15/09/2014.

Use for importance classification:

The building's use is of type "establishment open to the public of 5th category" (Public establishment classification according to Article R*123-19 du code de la construction et de l'habitation)

According to Art. 2, Arrêté du 22/10/2010 modifié par Art.1, Arrêté du 15/09/2014, the importance category associated to seism is II.

Importance factor of the building

The factor γ_I is function of the importance category of the building. Its value of 1 is specified in Art. 2 §III, Arrêté du 22/10/2010 modified by Art.1, Arrêté du 15/09/2014.

Ground type

The ground of construction is of type "C" :

Deep deposits of dense or medium-dense sand, gravel or stiff clay with thickness from several tens to many hundreds of metres

Horizontal design acceleration for a ground of type "C"

The ground acceleration intensity is calculated as follows:

$$a_g S = (a_{gr} \cdot \gamma_I) \cdot S \quad (\text{NF EN 1998-1 §3.2.2.2(1)})$$

$$a_g S = (1.6 \cdot 1.0) \cdot 1.5$$

$$a_g S = 2.400 \text{ m/s}^2$$

Paraseismic checking

A building located in seismic zone 4 and belonging to the important class II requires the mandatory application of the Eurocode 8.

Horizontal design spectrum for elastic analysis

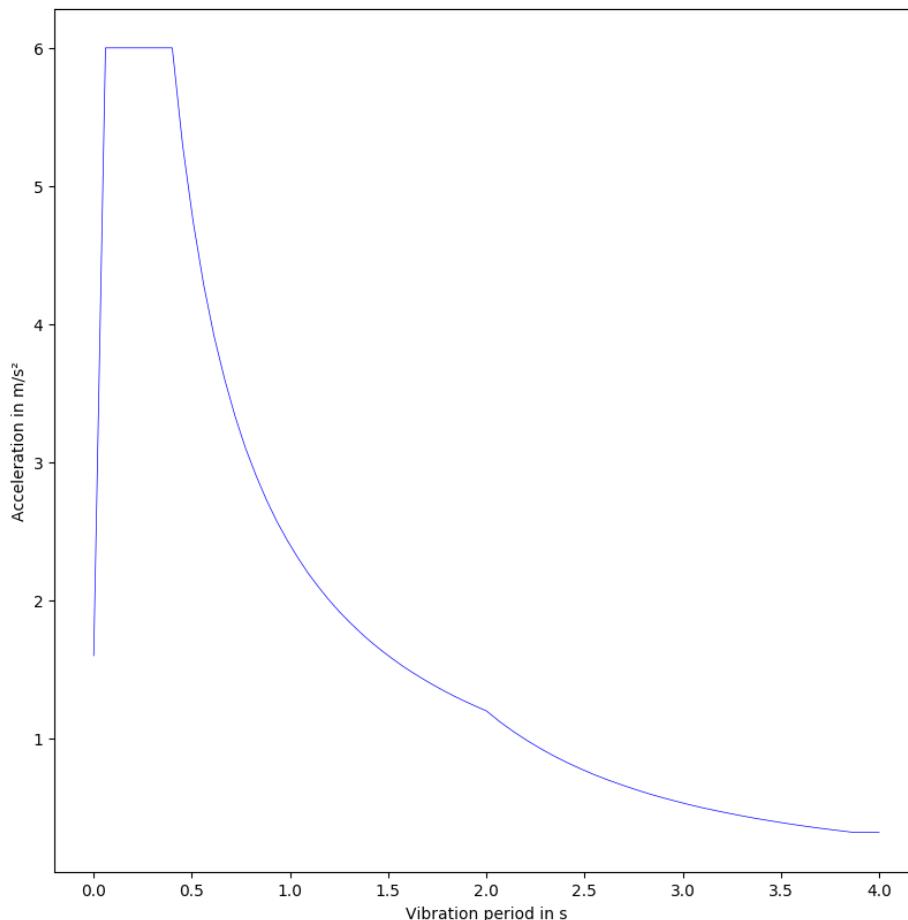
The elastic analysis is performed using a horizontal design spectrum of Type 2 - modified by Art.4 Arrêté du 22/10/2010.

It provides the maximum acceleration of the structure anchored to the ground during an earthquake. With, according to EN 1998-1 §3.2.2.5:

- the parameters of the design spectrum, function of the ground type and the seismicity zone:
 - T is the vibration period of a linear single-degree-of-freedom system.
 - the lower limit of the period of the constant spectral acceleration branch : $T_B = 0.06 \text{ s}$.



- the upper limit of the period of the constant spectral acceleration branch : $T_C = 0.4$ s.
- the value defining the beginning of the constant displacement response range of the spectrum : $T_D = 2$ s.
- the lower bound factor for the horizontal design spectrum : $\beta = 0.2$.
- the design ground acceleration, taking into account the lithologic effects of site that amplify the seismic movement depending on the nature of the basement : $a_g S = 2.40 \text{ m/s}^2$.
- behavior factor to take into account the energy dissipation capacity of the structure : $q = 1$.
- $S_d(T)$ the horizontal design spectrum defined by the following expressions :
 - $0 \leq T \leq T_B : S_d(T) = a_g \cdot S \cdot \left[\frac{2}{3} + \frac{T}{T_B} \cdot \left(\frac{2.5}{q} - \frac{2}{3} \right) \right]$ (EN 1998-1 equation 3.13)
 - $T_B \leq T \leq T_C : S_d(T) = a_g \cdot S \cdot \frac{2.5}{q}$ (EN 1998-1 equation 3.14)
 - $T_C \leq T \leq T_D : S_d(T) = \max \left(a_g \cdot S \cdot \frac{2.5}{q} \cdot \left[\frac{T_C}{T} \right], \beta \cdot a_g \right)$ (EN 1998-1 equation 3.15)
 - $T_D \leq T : S_d(T) = \max \left(a_g \cdot S \cdot \frac{2.5}{q} \cdot \left[\frac{T_C \cdot T_D}{T^2} \right], \beta \cdot a_g \right)$ (EN 1998-1 equation 3.16)



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Design ground displacement

The design ground displacement is evaluated to

$$d_g = 0.025 \cdot a_g \cdot S \cdot T_C \cdot T_D \quad (\text{EN 1998-1 equation 3.12})$$

$$d_g = 0.025 \cdot 1.6 \cdot 1.5 \cdot 0.4 \cdot 2.0$$

$$d_g = 4.8 \text{ cm.}$$

Vertical design acceleration

The intensity of the design acceleration of the ground in the vertical direction is calculated as :

$$a_{vg} = a_g \cdot \left[\frac{A_{vg}}{A_g} \right] \quad (\text{Art. 4 II c), Arrêté du 22/10/2010})$$

$$a_{vg} = 1.6 \cdot [0.9]$$

$$a_{vg} = 1.440 \text{ m/s}^2$$

Vertical design spectrum for elastic analysis

The vertical design acceleration is less than 2.5m/s^2 , its influence is not to be considered. (EN 1998-1 §4.3.3.5.2)

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